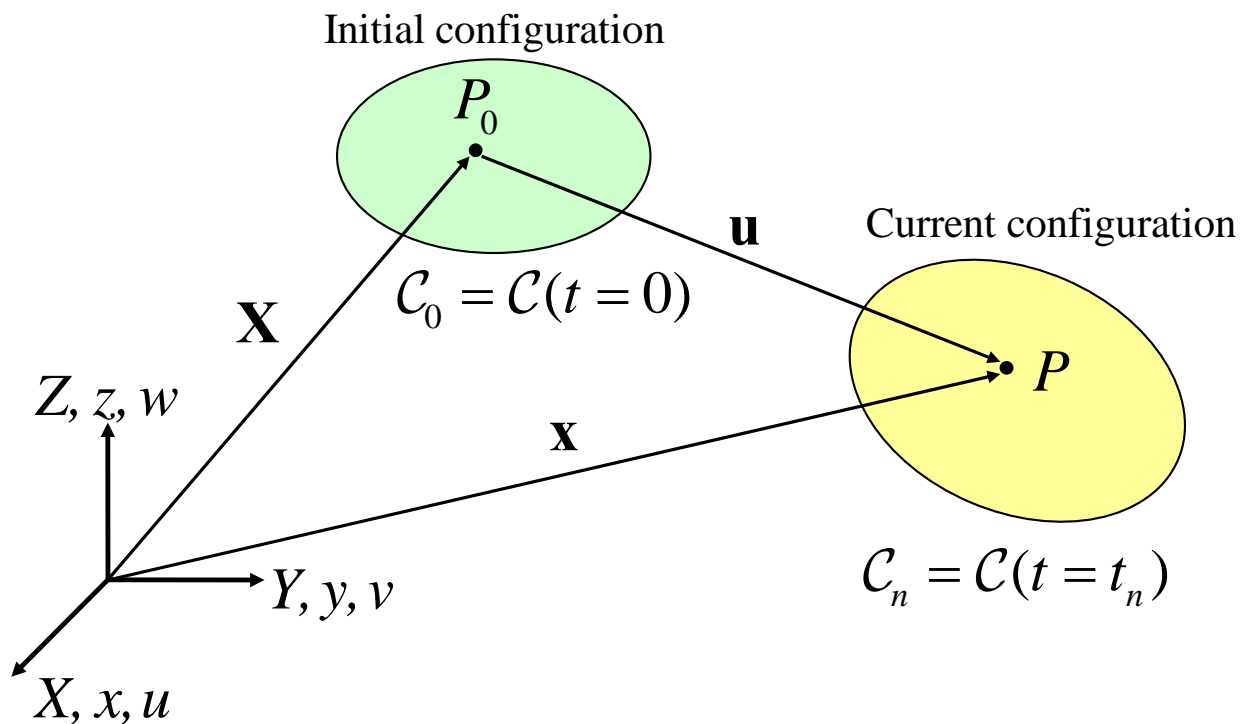


# Lecture 12:

## Formulation of Geometrically Nonlinear FE

### Review of Continuum Mechanics

- In the following the necessary background in the theory of the mechanics of continuous media (*continuum mechanics*) for derivation of *geometrically nonlinear finite elements* is presented
- In continuum mechanics a solid *structure* is mathematically *treated as a continuum body* being formed by a set of material particles
- The *position of all material particles* comprising the body at a given time  $t$  is *called* the *configuration* of the body, and denoted  $\mathcal{C}$
- A *sequence of configurations* for all times  $t$  *defines* the *motion of the body*
- In previous lectures we have seen that the motion of a body or structure is often represented by a load-displacement diagram, starting from an initial, usually undeformed, state at time  $t = 0$ , called *initial configuration*,  $\mathcal{C}_0$ , to which displacements  $\{\mathbf{u}\}$  are referred
- Each individual point on the equilibrium path corresponds to an instantaneous actual or *current* (deformed) *configuration*,  $\mathcal{C}_n$ , at time  $t = t_n$



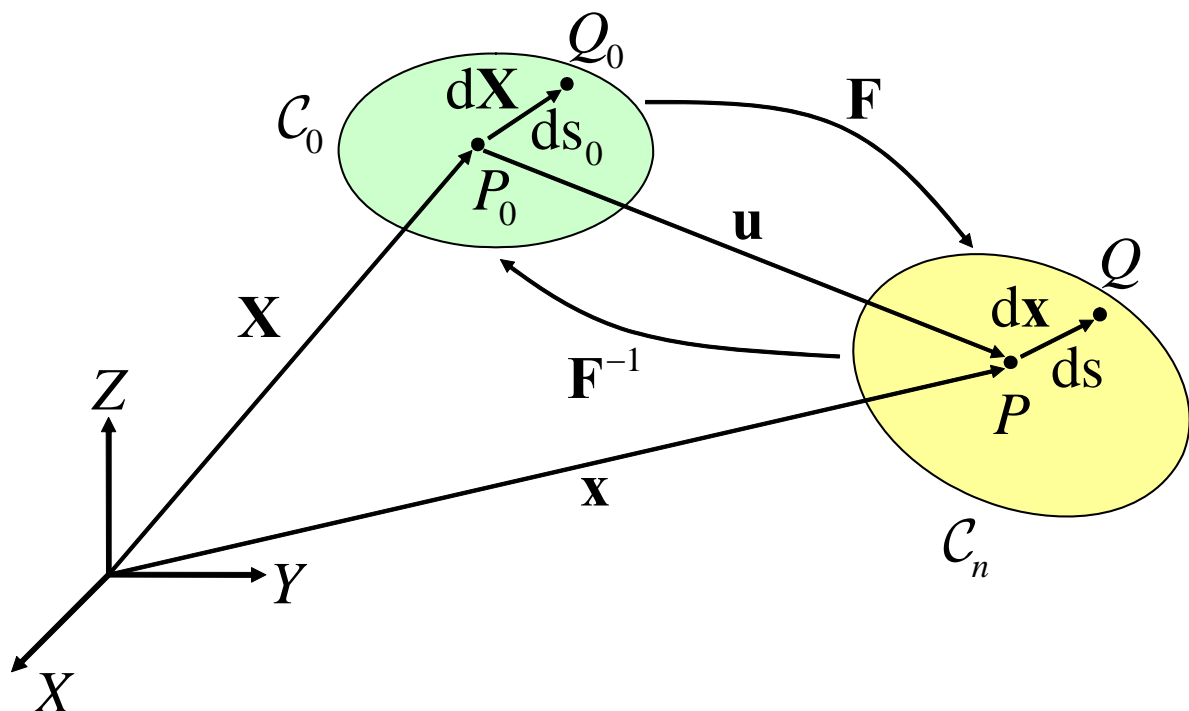
- The *reference configuration*, is the *configuration to which state variables* (e.g. strains and stresses) *are referred*
- It is important to *note that* the *time  $t$*  is not necessarily the physical time; in this context  $t$  *should be viewed as a state or load parameter* or simply a pseudotime  $\lambda$
- Three *basic choices* need to be made *in* developing a *large displacement (deformation) analysis scheme*:
  1. The *kinematic description*; i.e. how the body move and how the local deformations and strains are measured
  2. The *balance law*; i.e. the definition of linear and angular momentum and the definition of (conjugate) stresses
  3. The *constitutive equations*; i.e. an appropriate material relation that is objective and defines the stresses in terms of strains or rate of strains

## Description of Motion:

- To describe the deformation of a body requires knowledge of the *position occupied by the material particles* comprising the body *at all time*
- Two sets of coordinates may be used:
  - i) *Material (Lagrangian) coordinates*;  $\{\mathbf{X}\}$
  - ii) *Spatial (Eulerian) coordinates*;  $\{\mathbf{x}\} = \{\mathbf{x}(\mathbf{X}, t)\}$
- $\{\mathbf{x}\}$  defines the *current coordinates of material particles* in terms of *material coordinates*  $\{\mathbf{X}\}$ , the latter *being the initial coordinates of the particles* at time  $t = 0$
- In the *Lagrangian approach*, all physical *quantities* (displacements, strains and stresses) are *expressed as functions of time  $t$  and their initial position*  $\{\mathbf{X}\}$ , in the *Eulerian approach* they are *functions of time and their current position*
- Although both approaches may be used, the *Lagrangian approach* turns out to be the *most attractive in solid and structural mechanics* problems
- The *Lagrangian description of motion* is *referred to a fixed global, Cartesian coordinate system*  $(X, Y, Z)$
- In the Lagrangian description *displacements of any material point* in the solid is given by:

$$\{\mathbf{x}(\mathbf{X}, t)\} = \{\mathbf{X}\} + \{\mathbf{u}(\mathbf{X}, t)\} \quad \Leftrightarrow \quad \{\mathbf{u}(\mathbf{X}, t)\} = \{\mathbf{x}(\mathbf{X}, t)\} - \{\mathbf{X}\}$$

## Deformation Gradient and Strain Measures:



- In order to define the strain we need to know the *relative motion of two neighbouring particles*. Two such particles ( $P$  and  $Q$ ) are shown in the Figure above where at time  $t = 0$  the relative position is  $\{d\mathbf{X}\}$  and at time  $t = t_n$  the relative position is  $\{d\mathbf{x}\}$
- The *deformation gradient*  $[\mathbf{F}]$ , *describes* the *mapping* (deformation) *of* the *infinitesimal material 'fibre'*  $\{d\mathbf{X}\}$ , with length  $ds_0$ , *in*  $\mathcal{C}_0$  (the initial configuration) *to its new position*  $\{d\mathbf{x}\}$ , with length  $ds$ , *in*  $\mathcal{C}_n$  (the current configuration):

$$\{d\mathbf{x}\} = [\mathbf{F}]\{d\mathbf{X}\}$$

where

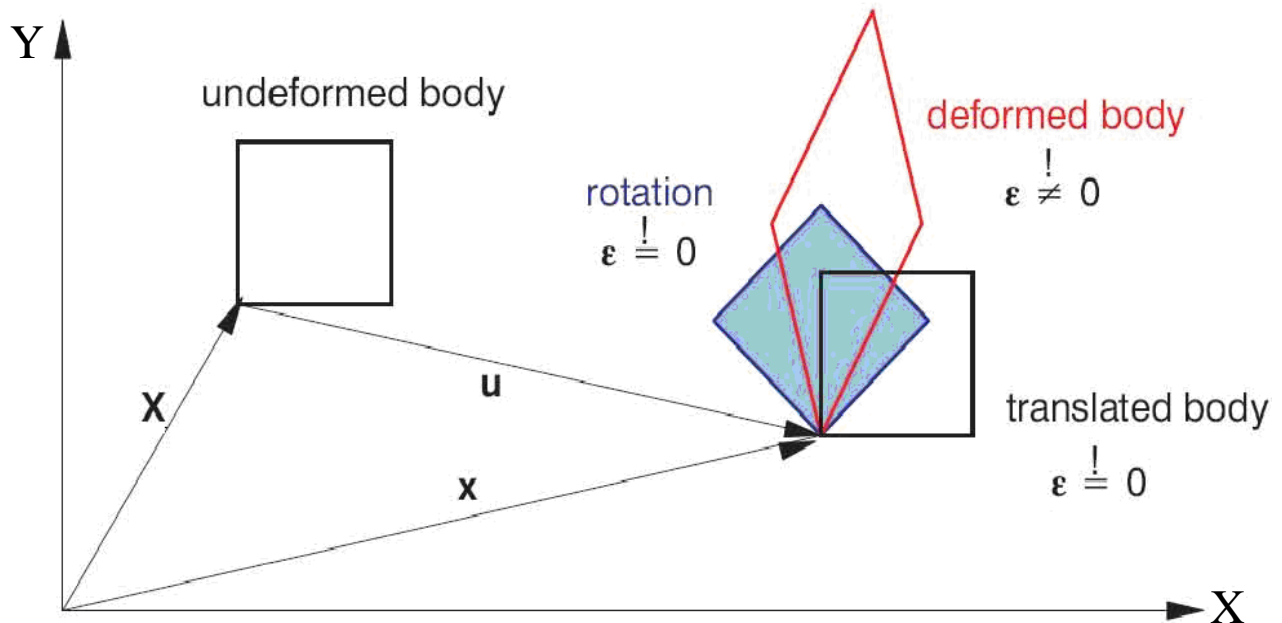
$$[\mathbf{F}] = \left[ \frac{\partial \mathbf{x}}{\partial \mathbf{X}} \right] = \left[ \frac{\partial (\mathbf{X} + \mathbf{u})}{\partial \mathbf{X}} \right] = [\mathbf{I}] + \left[ \frac{\partial \mathbf{u}}{\partial \mathbf{X}} \right] = [\mathbf{I}] + [\mathbf{G}]$$

$[\mathbf{I}]$  is the *unit tensor* and  $[\mathbf{G}]$  is called the *displacement gradient tensor*

- The *components of the deformation gradient*  $[\mathbf{F}]$ , and the *displacement gradient tensor*  $[\mathbf{G}]$ , thus becomes:

$$[\mathbf{F}] = \begin{bmatrix} \frac{\partial x}{\partial X} & \frac{\partial x}{\partial Y} & \frac{\partial x}{\partial Z} \\ \frac{\partial y}{\partial X} & \frac{\partial y}{\partial Y} & \frac{\partial y}{\partial Z} \\ \frac{\partial z}{\partial X} & \frac{\partial z}{\partial Y} & \frac{\partial z}{\partial Z} \end{bmatrix} \quad \text{and} \quad [\mathbf{G}] = \begin{bmatrix} \frac{\partial u}{\partial X} & \frac{\partial u}{\partial Y} & \frac{\partial u}{\partial Z} \\ \frac{\partial v}{\partial X} & \frac{\partial v}{\partial Y} & \frac{\partial v}{\partial Z} \\ \frac{\partial w}{\partial X} & \frac{\partial w}{\partial Y} & \frac{\partial w}{\partial Z} \end{bmatrix}$$

- The *deformation gradient*  $[\mathbf{F}]$  *describes stretches and rigid body motion of the material fibers from*  $\mathcal{C}_0$  *to*  $\mathcal{C}_n$
- In contrast to a linear analysis, where we may apply a linear strain measure (e.g. the engineering strain), *a finite strain measure* is used to *represent local deformations in a large deformation nonlinear analysis*
- In large deformation nonlinear analysis, a body may be subjected to both large rigid body motion and large deformations*  
 $\Rightarrow$  *An important feature of a finite strain measure is that it vanish for arbitrary rigid body translations and rotations*
- Another property of the *finite strain measure* is that it *must reduce to the infinitesimal strains if it is linearized* (i.e. when the nonlinear strain terms are neglected)



- One finite strain measure that has these desired properties is the **Green strain tensor**  $[\boldsymbol{\varepsilon}_G]$ , which is a **symmetric tensor** defining the **relationship between the squares of the length of the material 'fibre' vector**  $\{d\mathbf{X}\}$  with length  $ds_0$  in  $\mathcal{C}_0$  to its **deformed vector**  $\{d\mathbf{x}\}$  with length  $ds$  in  $\mathcal{C}_n$ :

$$ds^2 - ds_0^2 = 2\{d\mathbf{X}\}^T [\boldsymbol{\varepsilon}_G] \{d\mathbf{X}\}$$

- **Green strain tensor**  $[\boldsymbol{\varepsilon}_G]$  can also be **expressed in terms of the deformation gradient**  $[\mathbf{F}]$  through:

$$[\boldsymbol{\varepsilon}_G] = \frac{1}{2}([\mathbf{F}]^T [\mathbf{F}] - [\mathbf{I}])$$

with components:

$$[\boldsymbol{\varepsilon}_G] = \begin{bmatrix} \varepsilon_{GXX} & \varepsilon_{GXY} & \varepsilon_{GXZ} \\ \varepsilon_{GYX} & \varepsilon_{GYY} & \varepsilon_{GYZ} \\ \varepsilon_{GZX} & \varepsilon_{GZY} & \varepsilon_{GZZ} \end{bmatrix}$$

- The six strain *components of the Green strain tensor* may be expressed in terms of the displacement gradients:

$$\begin{aligned}\varepsilon_{GXX} &= \frac{\partial u}{\partial X} + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial X} \right)^2 + \left( \frac{\partial v}{\partial X} \right)^2 + \left( \frac{\partial w}{\partial X} \right)^2 \right] \\ \varepsilon_{GYX} &= \frac{\partial v}{\partial Y} + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial Y} \right)^2 + \left( \frac{\partial v}{\partial Y} \right)^2 + \left( \frac{\partial w}{\partial Y} \right)^2 \right] \\ \varepsilon_{GZX} &= \frac{\partial w}{\partial Z} + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial Z} \right)^2 + \left( \frac{\partial v}{\partial Z} \right)^2 + \left( \frac{\partial w}{\partial Z} \right)^2 \right] \\ \varepsilon_{GXY} &= \frac{1}{2} \left( \frac{\partial u}{\partial Y} + \frac{\partial v}{\partial X} \right) + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial X} \right) \left( \frac{\partial u}{\partial Y} \right) + \left( \frac{\partial v}{\partial X} \right) \left( \frac{\partial v}{\partial Y} \right) + \left( \frac{\partial w}{\partial X} \right) \left( \frac{\partial w}{\partial Y} \right) \right] \\ \varepsilon_{GYZ} &= \frac{1}{2} \left( \frac{\partial v}{\partial Z} + \frac{\partial w}{\partial Y} \right) + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial Y} \right) \left( \frac{\partial u}{\partial Z} \right) + \left( \frac{\partial v}{\partial Y} \right) \left( \frac{\partial v}{\partial Z} \right) + \left( \frac{\partial w}{\partial Y} \right) \left( \frac{\partial w}{\partial Z} \right) \right] \\ \varepsilon_{GZX} &= \frac{1}{2} \left( \frac{\partial w}{\partial X} + \frac{\partial u}{\partial Z} \right) + \frac{1}{2} \left[ \left( \frac{\partial u}{\partial Z} \right) \left( \frac{\partial u}{\partial X} \right) + \left( \frac{\partial v}{\partial Z} \right) \left( \frac{\partial v}{\partial X} \right) + \left( \frac{\partial w}{\partial Z} \right) \left( \frac{\partial w}{\partial X} \right) \right]\end{aligned}$$

- Green strain tensor* is *symmetric*:

$$\Rightarrow \varepsilon_{GYX} = \varepsilon_{GXY}, \quad \varepsilon_{GZY} = \varepsilon_{GYZ} \quad \text{and} \quad \varepsilon_{GXZ} = \varepsilon_{GZX}$$

- If the *nonlinear portion* (that enclosed in square brackets) is *neglected*, we obtain the *infinitesimal strains*:

$$\begin{aligned}\varepsilon_{xx} &= \varepsilon_{GXX}, & \varepsilon_{yy} &= \varepsilon_{GYX}, & \varepsilon_{zz} &= \varepsilon_{GZX} \\ \gamma_{xy} &= 2\varepsilon_{GXY}, & \gamma_{yz} &= 2\varepsilon_{GYZ}, & \gamma_{zx} &= 2\varepsilon_{GZX}\end{aligned}$$

- Green strain tensor* is often *used for problems with large displacements but small strains*

- *Several other finite strain measures are used in nonlinear continuum mechanics*, however, *they all have to satisfy the constraints* of finite strain measures:
  - They *must predict zero strains for arbitrarily rigid-body motions*, and
  - They *must reduce to the infinitesimal strains if the nonlinear terms are neglected*
- *For the uniaxial case of a stretched bar* that has *initial length  $L_0$  in  $C_0$  and length  $L$  in  $C_n$* , the *Green strain becomes*:

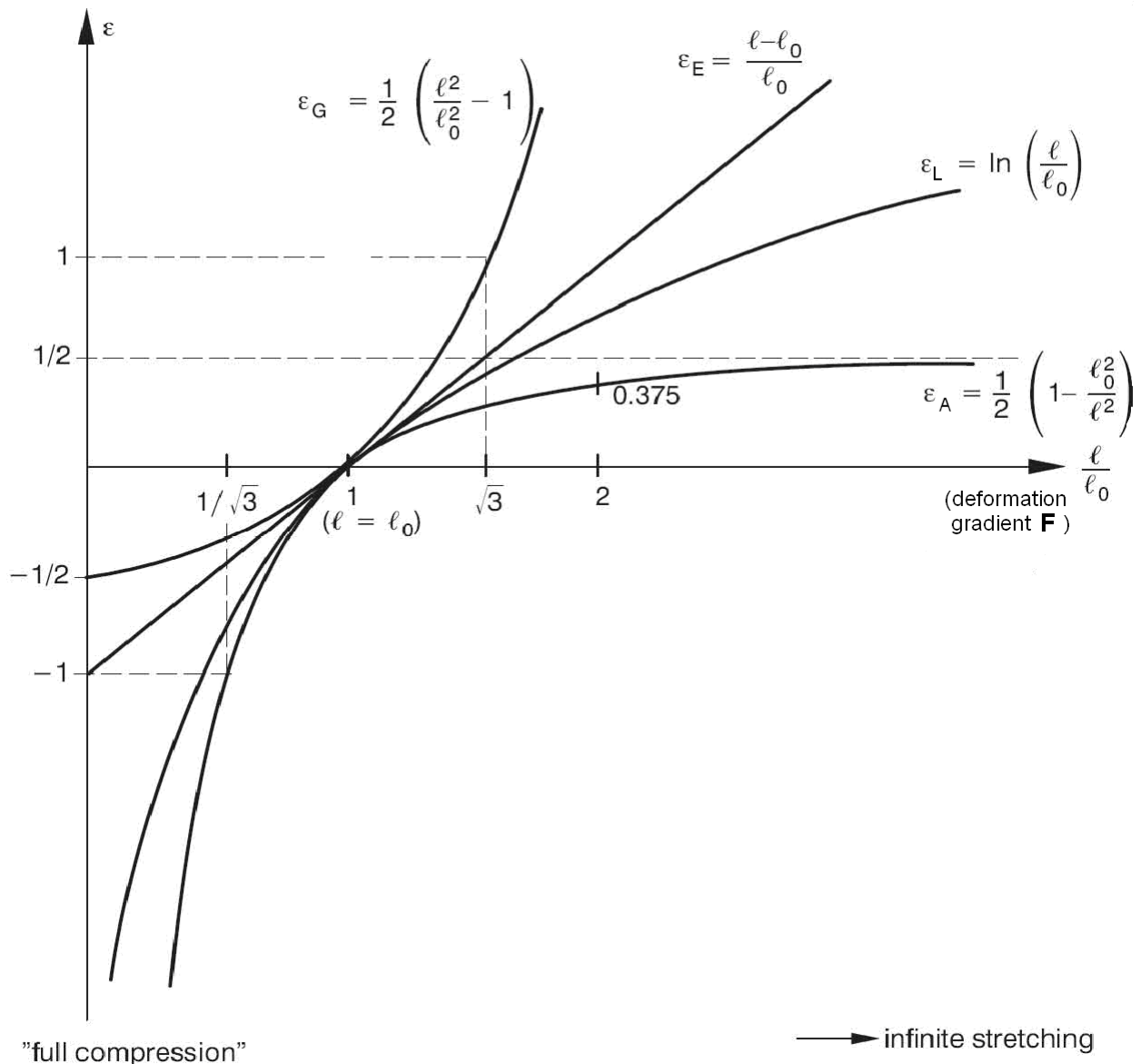
$$\varepsilon_G = \varepsilon_{GXX} = \frac{L^2 - L_0^2}{2L_0^2}$$

- Other *uniaxial strain measures* that are *frequently used* in nonlinear structural and solid mechanics:

Almansi strain:	$\varepsilon_A = \frac{L^2 - L_0^2}{2L^2}$
Logarithmic strain:	$\varepsilon_L = \log\left(\frac{L}{L_0}\right)$
Engineering strain:	$\varepsilon_E = \frac{L - L_0}{L_0}$

- *Almansi strains* are, in contrast to the Green strains that are referred to the material coordinates  $\{\mathbf{X}\}$ , *referred to the spatial coordinates  $\{\mathbf{x}\}$  and used together with an Euler description*, while *logarithmic* (also called natural or “true”) *strains* are *useful for large strain problems* (e.g. metal forming)





- When choosing *a proper finite strain measure* we have to judge whether the strain measure *predicts a realistic finite strain value* or not
- *If we want to model large strain deformations*, the *chosen strain measure should tend to  $-\infty$  for "full compression" and  $\infty$  for "infinite stretching"*, otherwise it could become difficult to describe a sensible constitutive law
- In the *Figure above* that *shows the behaviour of* the different *strain measures* introduced *for large strains*, we observe that

both the *Green* and the *Engineering strains* remain finite for “infinite” compression, while the *Almansi strain* predicts a finite strain for “infinite” tension

⇒ The *only strain measure* which is *suitable in* the *entire range* is the *logarithmic* (natural) *strain*

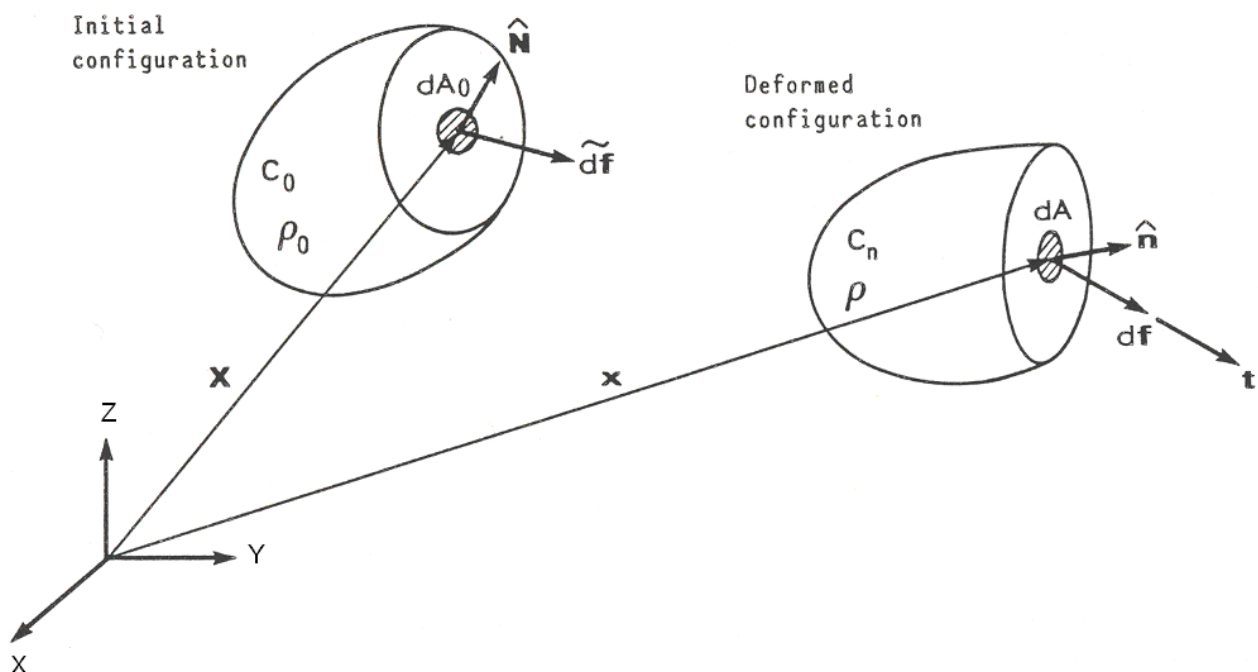
- However, if  $\left| \frac{L - L_0}{L_0} \right| < 0.05$  the *deviation between* the *finite strain measures* and the *Engineering strain* is of the order 2-3%

## Stress Measures:

- The *surface traction*  $\{\mathbf{t}\}$  is defined as:

$$\{\mathbf{t}\} = \frac{\{d\mathbf{f}\}}{dA}$$

where  $\{d\mathbf{f}\}$  is the infinitesimal force vector that acts on the infinitesimal area element  $dA$  in deformed configuration.



- The *Cauchy* or *true stress tensor*  $[\boldsymbol{\sigma}]$ , energy conjugate to the Almansi strain tensor  $[\boldsymbol{\varepsilon}_A]$ , gives the current force per unit area in deformed configuration, consequently:

$$\{\mathbf{t}\} = [\boldsymbol{\sigma}]\{\hat{\mathbf{n}}\}$$

where  $\{\hat{\mathbf{n}}\}$  is the unit outward normal to the infinitesimal area element  $dA$  in deformed configuration.

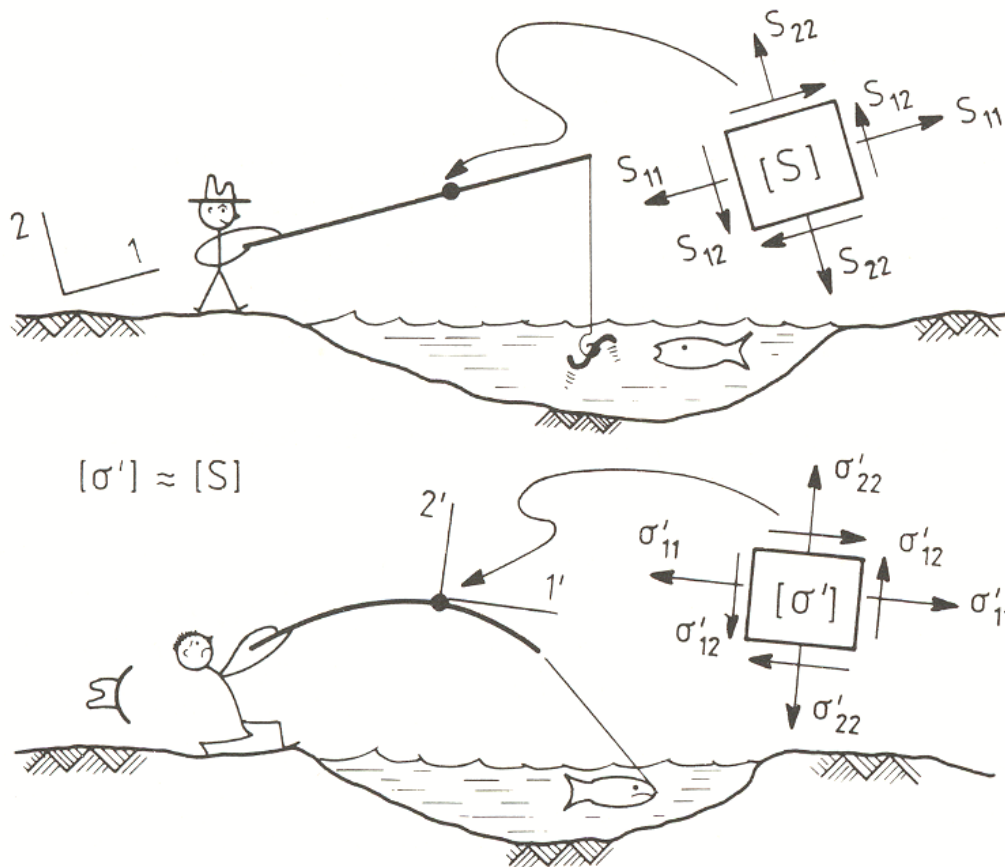
- Multiplying  $[\boldsymbol{\sigma}]$  by the determinant of  $[\mathbf{F}]$  ( $J = \det[\mathbf{F}]$ ) gives the *Kirchhoff stress tensor*  $[\boldsymbol{\tau}]$

$$[\boldsymbol{\tau}] = J[\boldsymbol{\sigma}]$$

- A *stress tensor work conjugate to* the *Green strain tensor*  $[\boldsymbol{\varepsilon}_G]$  must be referred to the initial (undeformed) configuration as is the Green strain tensor.
- It may be shown that the *2<sup>nd</sup> Piola-Kirchhoff (PK) stress tensor*  $[\mathbf{S}]$  that gives the transformed current force  $\{d\tilde{\mathbf{f}}\}$  per unit undeformed area  $dA_0$  is work conjugate to  $[\boldsymbol{\varepsilon}_G]$  and related to  $[\boldsymbol{\sigma}]$  through

$$[\mathbf{S}] = J[\mathbf{F}]^{-1}[\boldsymbol{\sigma}][\mathbf{F}]^{-T}$$

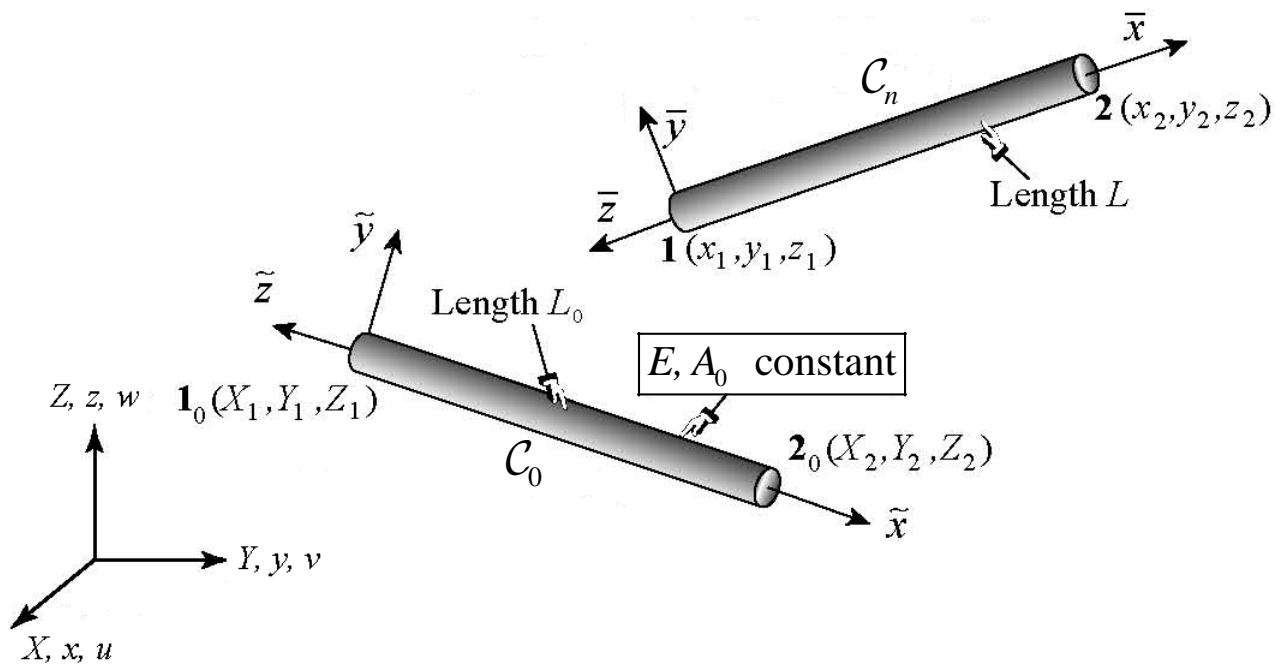
- While the *Cauchy stress tensor*  $[\boldsymbol{\sigma}]$  and the *Kirchhoff stress tensor*  $[\boldsymbol{\tau}]$  are *preferable in general NFEA involving large deformations*, the *2<sup>nd</sup> PK stress tensor*  $[\mathbf{S}]$  is *a good approximation when the deformational (strain giving) displacement components are small* (i.e. large rigid body displacements, but small strains).



## Total and Updated Lagrangian Formulations:

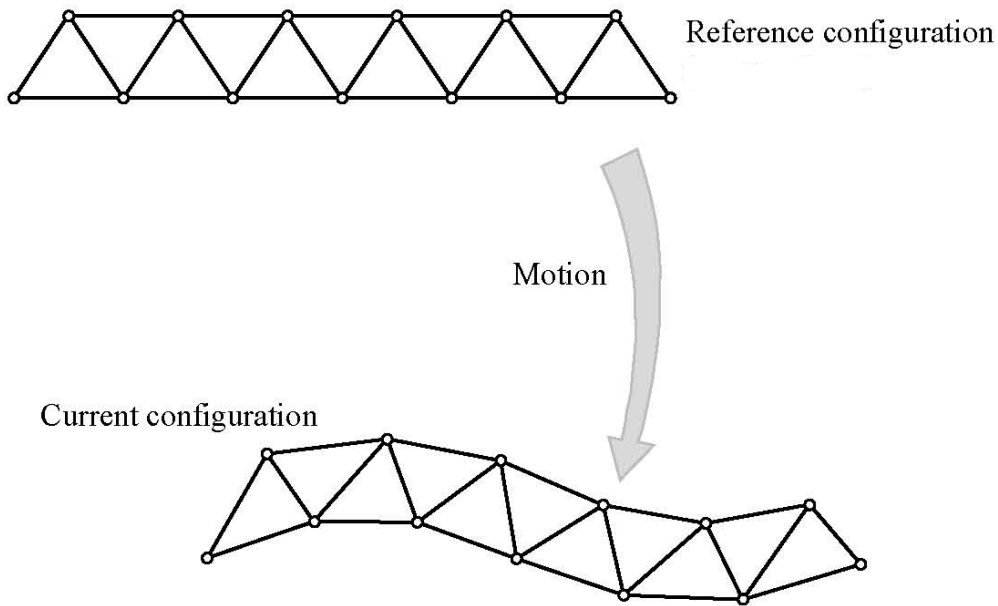
- In a **Total Lagrangian (TL) formulation** strain and stress measures are referred to the **initial** (undeformed) configuration,  $C_0$
- Alternatively if a known **deformed configuration**,  $C_n$ , is taken as the initial state and continuously updated as the calculation proceeds this is called an **Updated Lagrangian (UL) formulation**
- In a **CoRotational (CR) formulation** a local reference frame,  $C_R$ , is attached to each element and translates and rotates with the element as a rigid body. In a CR formulation, the total deformation is decomposed into a rigid-body motion, which is identical to rigid-body motion of the local reference frame, and local deformations (strains and stresses), that are measured relative to the local reference frame

## 2-node TL Bar Element in 3D Space<sup>1</sup>



- In the following the *key concepts of nonlinear continuum mechanics* are *applied to establish* the *internal forces*  $\{\mathbf{r}^{\text{int}}\}$  *and tangential stiffness*  $[\mathbf{k}_t]$  of a 2-node three dimensional bar element based on the Total Lagrangian formulation
- The 2-node bar element may be used to model truss structures as shown in the Figure on the next page
- It is assumed that the *material behaviour* is *linearly elastic* with elasticity modulus  $E$ , *such that we may consider geometric nonlinear effects only*
- *In* the initial configuration  $C_0$ , *which is the reference configuration for the TL formulation*, the *element has cross section area*  $A_0$  (assumed constant along the element) *and length*  $L_0$
- *In* the current configuration  $C_n$ , the *cross section area and length* become  $A$  *and*  $L$ , respectively

<sup>1</sup> Carlos Felippa, University of Colorado at Boulder: Chapter 14 of lecture notes in ASEN 5107 (NFEM).



## Element Kinematics:

- Assume that the *bar remains straight* in any configuration  
 $\Rightarrow$  The *coordinates of a generic point*  $\{\mathbf{X}\}$  *located on the longitudinal axis of the reference configuration*  $\mathcal{C}_0$  *and the corresponding coordinates*  $\{\mathbf{x}\}$  *in the current configuration*  $\mathcal{C}_n$ , reads:

$$\{\mathbf{X}(\xi)\} = \begin{Bmatrix} X(\xi) \\ Y(\xi) \\ Z(\xi) \end{Bmatrix} = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 \\ 0 & N_1 & 0 & 0 & N_2 & 0 \\ 0 & 0 & N_1 & 0 & 0 & N_2 \end{bmatrix} \begin{Bmatrix} X_1 \\ Y_1 \\ Z_1 \\ X_2 \\ Y_2 \\ Z_2 \end{Bmatrix} = [\mathbf{N}]\{\mathbf{C}\}$$

$$\{\mathbf{x}(\xi)\} = \begin{Bmatrix} x(\xi) \\ y(\xi) \\ z(\xi) \end{Bmatrix} = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 \\ 0 & N_1 & 0 & 0 & N_2 & 0 \\ 0 & 0 & N_1 & 0 & 0 & N_2 \end{bmatrix} \begin{Bmatrix} x_1 \\ y_1 \\ z_1 \\ x_2 \\ y_2 \\ z_2 \end{Bmatrix} = [\mathbf{N}]\{\mathbf{c}\}$$

where  $\xi$  is the *dimensionless* isoparametric *coordinate* that varies from  $\xi_1 = -1$  at node 1 to  $\xi_2 = 1$  at node 2, and  $N_i$  are the *linear shape functions*:

$$N_i = \frac{1}{2}(1 + \xi_i \xi); \quad i = 1, 2$$

- The *displacement field*,  $\{\mathbf{u}\}$ , is *obtained by subtracting* the two *position vectors*  $\{\mathbf{X}\}$  and  $\{\mathbf{x}\}$ :

$$\{\mathbf{u}(\xi)\} = \begin{Bmatrix} u(\xi) \\ v(\xi) \\ w(\xi) \end{Bmatrix} = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 \\ 0 & N_1 & 0 & 0 & N_2 & 0 \\ 0 & 0 & N_1 & 0 & 0 & N_2 \end{bmatrix} \begin{Bmatrix} u_1 \\ v_1 \\ w_1 \\ u_2 \\ v_2 \\ w_2 \end{Bmatrix} = [\mathbf{N}]\{\mathbf{d}\}$$

### Strain Energy:

- Denoting* the *axial strain and stress* measures by  $e$  and  $s$ , respectively, with  $s$  being the energy conjugate of  $e$
- Because of the *linear displacement* assumptions  
 $\Rightarrow$  *Strain  $e$  and stress  $s$  become constant* over the element length (volume)
- The *axial strain  $e$*  is *assumed to be zero in  $C_0$  and  $e$  in  $C_n$*   
 $\Rightarrow$  *The stresses in  $C_0$  and  $C_n$  become:*

$$\begin{array}{ll} s = s_0 & \text{in } C_0 \\ s = s_0 + Ee & \text{in } C_n \end{array}$$

- Similarly, the *axial forces*  $N_0$  in  $C_0$  and  $N$  in  $C_n$  become:

$$\begin{array}{l} N_0 = A_0 s_0 \quad \text{in } C_0 \\ N = A_0 s = N_0 + EA_0 e \quad \text{in } C_n \end{array}$$

- The *strain energy density*  $U_0$  in  $C_0$  is *assumed to be zero* ( $e_0 = 0$ ), while *in*  $C_n$  it becomes:

$$U_0 = s_0 e + \frac{1}{2} E e^2$$

which is *constant over* the volume of the *element*

- The *total strain energy in*  $C_n$ , thus *becomes*:

$$U = \int_{V_0} U_0 dV = U_0 V_0 = A_0 L_0 \left( s_0 e + \frac{1}{2} E e^2 \right) = L_0 \left( N_0 e + \frac{1}{2} EA_0 e^2 \right)$$

## Internal Forces and Tangential Stiffness:

- The *FE equilibrium equations* are *obtained by making* the *total potential energy*  $U_0$  *stationary*

⇒ The *internal force vector*  $\{\mathbf{r}^{\text{int}}\}$  is *obtained as* the *gradient of* the *internal strain energy*  $U$  *with respect to* the *nodal displacements*  $\{\mathbf{d}\}$

$$\{\mathbf{r}^{\text{int}}\} = \left\{ \frac{\partial U}{\partial \mathbf{d}} \right\}$$



- It is assumed that the *strain measure*  $e$  is a *function of the element lengths*  $L_0$  in  $C_0$  and  $L$  in  $C_n$  (where  $L_0$  is fixed):

$$e = e(L) \quad \Rightarrow \quad U = U(e) = U(L)$$

- The *derivatives of the strain energy*  $U_0$  with respect to *nodal displacements*  $\{\mathbf{d}\}$  are *obtained by the chain rule*:

$$\left\{ \frac{\partial U}{\partial \mathbf{d}} \right\} = \frac{\partial U}{\partial e} \left\{ \frac{\partial e}{\partial \mathbf{d}} \right\} = L_0 (N_0 + EA_0 e) \left\{ \frac{\partial e}{\partial \mathbf{d}} \right\} = L_0 N \frac{\partial e}{\partial L} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}$$

- The *element length*  $L$  in  $C_n$  is defined by:

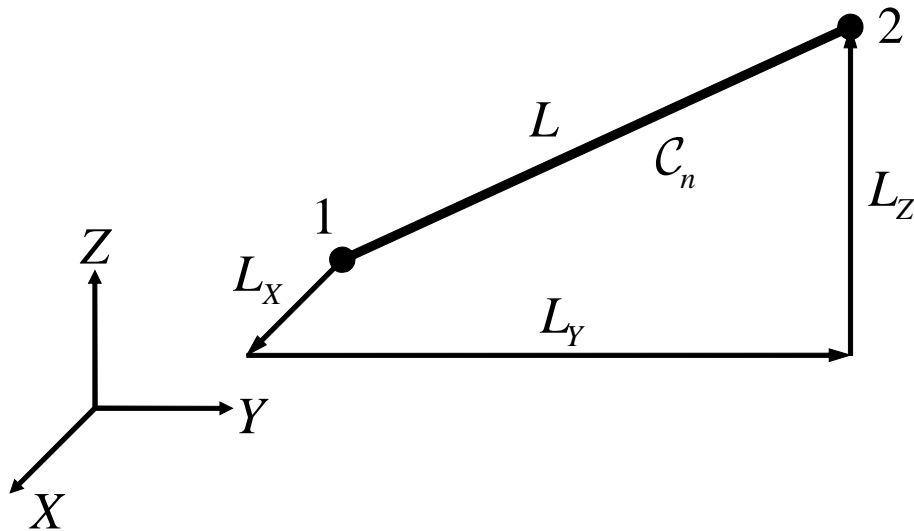
$$L = \sqrt{L_X^2 + L_Y^2 + L_Z^2}$$

where the *projected lengths onto the global axes in*  $C_n$  reads:

$$\begin{aligned} L_X &= x_2 - x_1 = (X_2 + u_2) - (X_1 + u_1) \\ L_Y &= y_2 - y_1 = (Y_2 + v_2) - (Y_1 + v_1) \\ L_Z &= z_2 - z_1 = (Z_2 + w_2) - (Z_1 + w_1) \end{aligned}$$

- The *partial derivatives of*  $L$  with respect to the *nodal displacements*  $\{\mathbf{d}\}$ , thus become:

$$\left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} = \begin{Bmatrix} \partial L / \partial u_1 \\ \partial L / \partial v_1 \\ \partial L / \partial w_1 \\ \partial L / \partial u_2 \\ \partial L / \partial v_2 \\ \partial L / \partial w_2 \end{Bmatrix} = \frac{1}{L} \begin{Bmatrix} -L_X \\ -L_Y \\ -L_Z \\ L_X \\ L_Y \\ L_Z \end{Bmatrix} = \begin{Bmatrix} -\mathbf{L} \\ \mathbf{L} \end{Bmatrix} = \{\hat{\mathbf{L}}\}$$



where  $\{\mathbf{L}\}$  contains the *direction cosines of the length segment  $L$* :

$$\{\mathbf{L}\} = \frac{1}{L} [L_x \quad L_y \quad L_z]^T$$

- Hence, the *internal force vector*  $\{\mathbf{r}^{\text{int}}\}$  may be *expressed in terms of the direction cosines* contained in  $\{\hat{\mathbf{L}}\}$ :

$$\{\mathbf{r}^{\text{int}}\} = \left\{ \frac{\partial U}{\partial \mathbf{d}} \right\} = L_0 N \frac{\partial e}{\partial L} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} = L_0 N \frac{\partial e}{\partial L} \{\hat{\mathbf{L}}\}$$

- Similarly, it may easily be shown that the *second derivatives of  $L$  with respect to the nodal displacements*  $\{\mathbf{d}\}$ , become:

$$\left[ \frac{\partial^2 L}{\partial \mathbf{d} \partial \mathbf{d}} \right] = \frac{1}{L} \left( \begin{bmatrix} \mathbf{I} & -\mathbf{I} \\ -\mathbf{I} & \mathbf{I} \end{bmatrix} - \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}^T \right) = \frac{1}{L} [\hat{\mathbf{I}} - \hat{\mathbf{L}} \hat{\mathbf{L}}^T]$$

- The *tangent stiffness*  $[\mathbf{k}_t]$  is *obtained* simply *by differentiating* the *internal force vector*  $\{\mathbf{r}^{\text{int}}\}$  *with respect to* the *nodal displacements*  $\{\mathbf{d}\}$ :

$$\begin{aligned}
[\mathbf{k}_t] &= \left\{ \frac{\partial \mathbf{r}^{\text{int}}}{\partial \mathbf{d}} \right\} \\
&= L_0 \left( \left\{ \frac{\partial N}{\partial \mathbf{d}} \right\} \frac{\partial e}{\partial L} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}^T + N \frac{\partial^2 e}{\partial L^2} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}^T + N \frac{\partial e}{\partial L} \left[ \frac{\partial^2 L}{\partial \mathbf{d} \partial \mathbf{d}} \right] \right) \\
&= L_0 \left( EA_0 \left( \frac{\partial e}{\partial L} \right)^2 \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}^T + N \left( \frac{\partial^2 e}{\partial L^2} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\} \left\{ \frac{\partial L}{\partial \mathbf{d}} \right\}^T + \frac{\partial e}{\partial L} \left[ \frac{\partial^2 L}{\partial \mathbf{d} \partial \mathbf{d}} \right] \right) \right)
\end{aligned}$$

- Substituting* the *expressions for the first and second partial derivatives of the element length from above*, we obtain:

$$\begin{aligned}
[\mathbf{k}_t] &= L_0 \left( EA_0 \left( \frac{\partial e}{\partial L} \right)^2 [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] + N \left( \frac{\partial^2 e}{\partial L^2} [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] + \frac{1}{L} \frac{\partial e}{\partial L} [\hat{\mathbf{I}} - \hat{\mathbf{L}} \hat{\mathbf{L}}^T] \right) \right) \\
&= L_0 \left( EA_0 \left( \frac{\partial e}{\partial L} \right)^2 [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] + N \left( \frac{1}{L} \frac{\partial e}{\partial L} [\hat{\mathbf{I}}] + \left( \frac{\partial^2 e}{\partial L^2} - \frac{1}{L} \frac{\partial e}{\partial L} \right) [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] \right) \right) \\
&= [\mathbf{k}_m] + [\mathbf{k}_g]
\end{aligned}$$

where the *material stiffness*  $[\mathbf{k}_m]$  *and* the *geometrical stiffness*  $[\mathbf{k}_g]$  reads:

$$\begin{aligned}
[\mathbf{k}_m] &= EA_0 L_0 \left( \frac{\partial e}{\partial L} \right)^2 [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] \\
[\mathbf{k}_g] &= NL_0 \left( \frac{1}{L} \frac{\partial e}{\partial L} [\hat{\mathbf{I}}] + \left( \frac{\partial^2 e}{\partial L^2} - \frac{1}{L} \frac{\partial e}{\partial L} \right) [\hat{\mathbf{L}} \hat{\mathbf{L}}^T] \right)
\end{aligned}$$

- The *above expressions for* the internal force vector  $\{\mathbf{r}^{\text{int}}\}$  *and* the tangent stiffness  $[\mathbf{k}_t]$  *are general and made independent of the choice of strain measure*
- The *appropriate choice of strain measure should be made to get the final form of* the internal force vector  $\{\mathbf{r}^{\text{int}}\}$  *and* tangent stiffness  $[\mathbf{k}_t]$
- *The values of the partial derivatives with respect to L and the final form of* the internal force vector  $\{\mathbf{r}^{\text{int}}\}$ , the material stiffness  $[\mathbf{k}_m]$ , *and* the geometric stiffness  $[\mathbf{k}_g]$  *for some specific strain measures are collected in the Table below:*

Strain Measure	$\frac{\partial e}{\partial L}$	$\frac{\partial^2 e}{\partial L^2}$	$\{\mathbf{r}^{\text{int}}\}$	$[\mathbf{k}_m]$	$[\mathbf{k}_g]$
$\varepsilon_E = \frac{L - L_0}{L_0}$	$\frac{1}{L_0}$	0	$N\{\hat{\mathbf{L}}\}$	$\frac{EA_0}{L_0}[\hat{\mathbf{L}}\hat{\mathbf{L}}^T]$	$\frac{N}{L}[\hat{\mathbf{I}} - \hat{\mathbf{L}}\hat{\mathbf{L}}^T]$
$\varepsilon_G = \frac{L^2 - L_0^2}{2L_0^2}$	$\frac{L}{L_0^2}$	$\frac{1}{L_0^2}$	$\frac{NL}{L_0}\{\hat{\mathbf{L}}\}$	$\frac{EA_0 L^2}{L_0^3}[\hat{\mathbf{L}}\hat{\mathbf{L}}^T]$	$\frac{N}{L_0}[\hat{\mathbf{I}}]$
$\varepsilon_L = \log\left(\frac{L}{L_0}\right)$	$\frac{1}{L}$	$-\frac{1}{L^2}$	$\frac{NL_0}{L}\{\hat{\mathbf{L}}\}$	$\frac{EA_0 L_0}{L^2}[\hat{\mathbf{L}}\hat{\mathbf{L}}^T]$	$\frac{NL_0}{L^2}[\hat{\mathbf{I}} - 2\hat{\mathbf{L}}\hat{\mathbf{L}}^T]$

- The *internal force vector and the geometric stiffness matrix for the Green strain measure* thus becomes:

$$\left\{ \mathbf{r}^{\text{int}} \right\} = \frac{N}{L_0} \begin{Bmatrix} -L_x \\ -L_y \\ -L_z \\ L_x \\ L_y \\ L_z \end{Bmatrix} \quad \text{and} \quad \left[ \mathbf{k}_g \right] = \frac{N}{L_0} \begin{bmatrix} 1 & 0 & 0 & -1 & 0 & 0 \\ 0 & 1 & 0 & 0 & -1 & 0 \\ 0 & 0 & 1 & 0 & 0 & -1 \\ -1 & 0 & 0 & 1 & 0 & 0 \\ 0 & -1 & 0 & 0 & 1 & 0 \\ 0 & 0 & -1 & 0 & 0 & 1 \end{bmatrix}$$